Roundabout Design and Capacity Analysis in Australia and New Zealand

International Roundabout Design and Capacity Seminar 6th International Symposium on Highway Capacity and Quality of Service, Stockholm, Sweden, 1 July 2011 Presenter: Rahmi Akçelik rahmi.akcelik@sidrasolutions.com



The objective of presentation

This presentation highlights various aspects of roundabouts in AUSTRALIA and NEW ZEALAND in response to the following objective of the International Roundabout Design and Capacity Seminar:

Present an overview of typical roundabout designs in urban and rural environment and treatments of bicycles and pedestrians, capacity and level-of-service methods used, and recent research on capacity and level-of-service.

This is by no means a complete account of the subject. For more detailed information, please see the publications referred to in the presentation as well as other papers and presentations available for download from:

www.sidrasolutions.com/software_downloads_articles.aspx

Also refer to related guidelines used in Australia and New Zealand.



ACKNOWLEDGEMENTS

AUSTRALIA

VicRoads: Douglas Harley, Matthew Hall

Queensland Department of Transport and Main Roads:

Owen Arndt, David Stewart, Robyn Davies, Mark McDonald

NEW ZEALAND

ViaStrada: Axel Wilke

Traffic Engineering Solutions: Ivan Jurisich



Australia And New Zealand





- Private car ownership is very high.
- Road use by commercial vehicles is substantial.
- Traffic congestion due to limited road space is a problem in large cities.

• Roundabouts are common and there are many. high-capacity roundabouts.





All-Way Stop Control is rare Two-Way Give Way (Yield) Control is common

This is likely to affect capacity of roundabouts





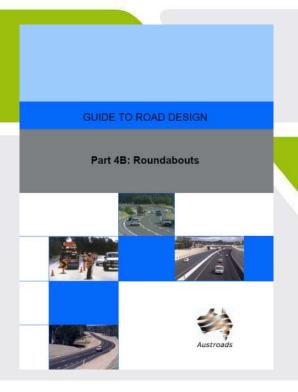


Roundabout design



AUSTROADS (2009) Guide to Road Design Part 4B - ROUNDABOUTS

New Roundabout Guide (replaces the 1993 AUSTROADS Guide to Traffic Engineering, Part 6)



1.9 Significant Change from the Guide to Traffic Engineering Practice – Part 6: Roundabouts

A most significant change in this guide compared to the previous Austroads guide is that deflection is no longer used as a fundamental parameter in achieving control of the speed of vehicles at roundabouts. The method in this guide controls the speed of traffic entering roundabouts through the geometry of the roundabout entry, rather than within the roundabout where restriction through deflection requirements is essentially too late in the process of the driver negotiating the roundabout.

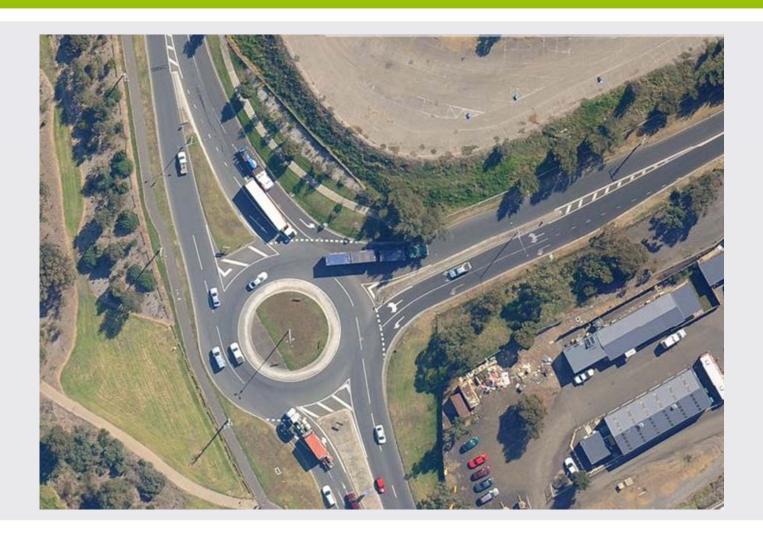


















Roundabout examples





Roundabout examples





Roundabout examples – bus bypass lanes







Roundabout lane markings







Roundabouts in Australia – a survey of Australian professionals

Akçelik, R. (2008). Roundabouts in Australia. Paper presented at the National Roundabout Conference, Transportation Research Board, Kansas City, MO, USA, 18-21 May 2008.

Replacing signals with roundabouts or roundabouts with signals?

- Overall trend is to replace roundabouts with signals rather than the other way round. The reasons given include:
 - roundabout capacity limitations
 - unbalanced flow situations
 - better allowance for road users such as pedestrians and cyclists.



Roundabouts in Australia – a survey of Australian professionals

- A respondent commented: "Some see roundabouts as an evolutionary step before signals. A saturated signalised intersection is often treated through the construction of extra lanes, slip lanes and so on, rather than considering the option of a roundabout."
- Replacing a roundabout with signals due to capacity reasons applies particularly to two-lane roundabouts since it is not common to expand them to three-lane roundabouts to improve capacity (although it occasionally happens).

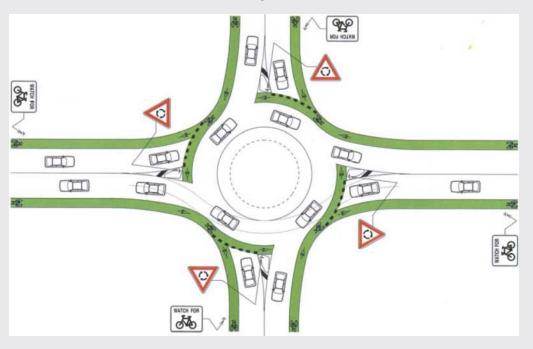


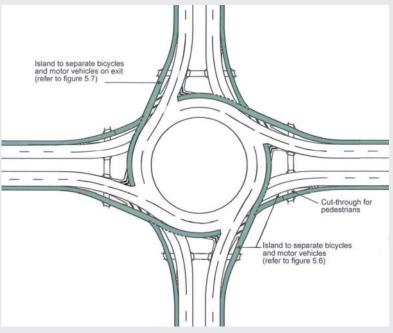
Bicycle and pedestrian treatment examples

Bicycle and pedestrian treatments

Refer to: AUSTROADS (2009) Guide to Road Design, Part 4B – ROUNDABOUTS, Section 5. This guide carries many warnings about possible treatments.

The official New Zealand Transport Agency guidance is not to use cycle lane markings within the roundabout. Refer to the New Zealand training material "Planning and Design for Cycling, Module 4, Section 4 – Roundabouts": http://viastrada.co.nz/sites/viastrada.co.nz/files/Module4-section4.pdf







Bicyle treatment examples





Bicyle treatment examples



Bicycle treatment examples



New Zealand



Bicycle treatment examples



New Zealand





New Zealand – The C-Roundabout

Paper by Ivan Jurisich presented at the TRB Roundabout Conference, Carmel, IN, USA, May 2011. For further information contact: Ivan.jurisich@tes.net.nz



VIDEOS are available.

Also refer to:

Multi-lane roundabout designs for cyclists. New Zealand Transport Agency, Research Report 287. Wellington, New Zealand, 2006.

www.nzta.govt.nz/resources/research/reports/287



New Zealand – The C-Roundabout





Capacity and Level of Service Methods



Capacity and Level of Service Methods

In Australia and New Zealand practice, SIDRA INTERSECTION software is the main method used for capacity, performance and level of service.

Information about the roundabout capacity model used in SIDRA INTERSECTION is given in the following slides.

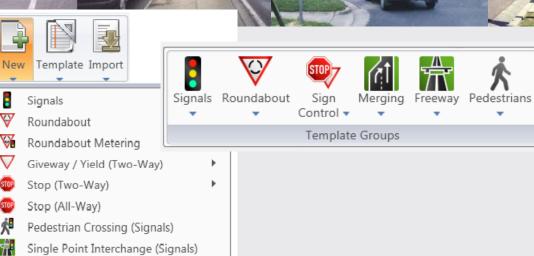


Features of SIDRA INTERSECTION

Model CONSISTENCY in evaluating alternative intersection treatments (e.g. Definition of delay, back of queue, stops, etc).



Total intersection analysis tool

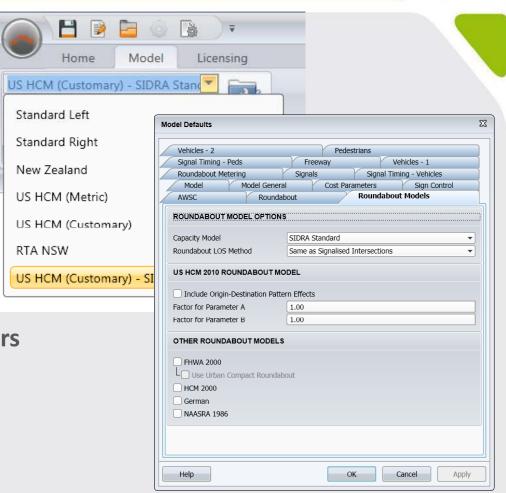




SIDRA models for LOCAL CONDITIONS

SIDRA INTERSECTION provides facilities to calibrate the traffic model for local traffic conditions:

- Standard models with various options are available for different countries.
- Customised Models can be prepared by users.
- Various options and key input parameters can be used for detailed calibration.
- HCM 2010 roundabout capacity model is fully integrated into the software.



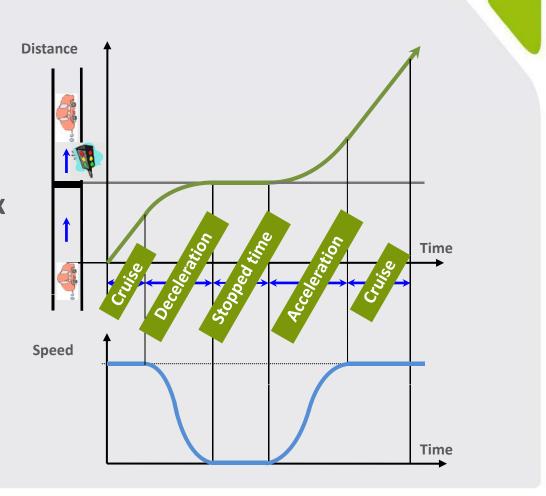


Features of SIDRA INTERSECTION

Power-based fuel and emission model

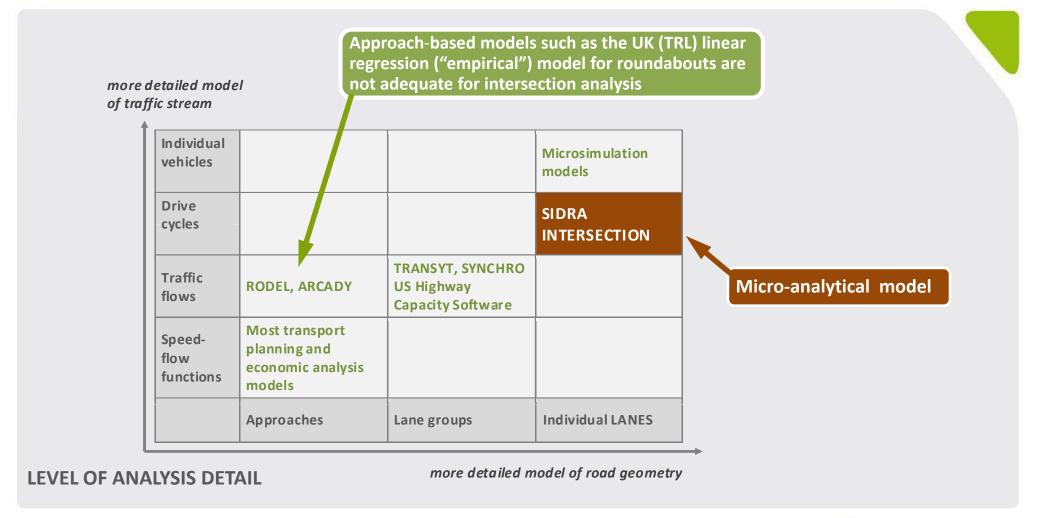
Four-mode elemental model (drive cycles)

- Emissions CO2, CO, HC, NOx
- Fuel consumption
- Operating COST



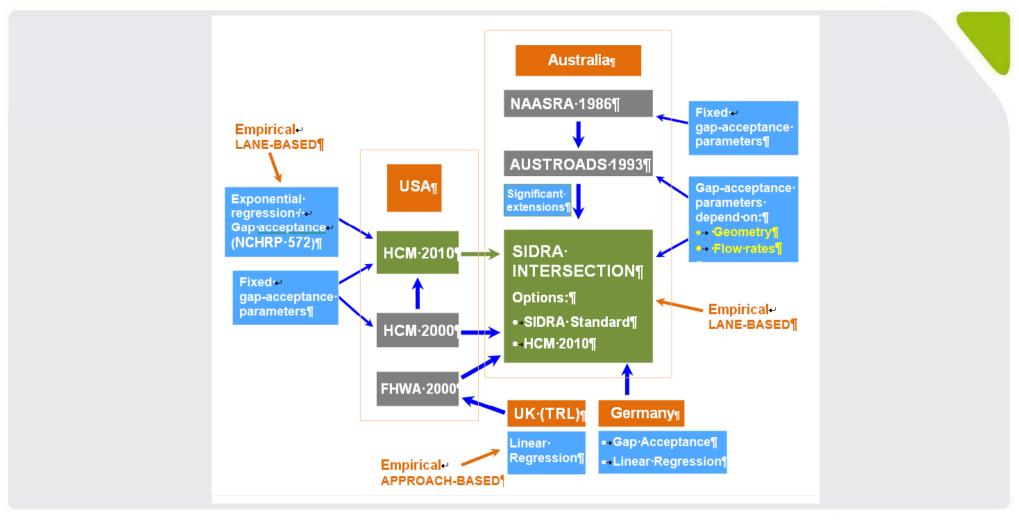


Features of SIDRA INTERSECTION





ROUNDABOUT MODELS in SIDRA INTERSECTION



Papers presented at the International Roundabout Conference, Transportation Research Board, Carmel, Indiana, USA, May 2011

- Some common and differing aspects of alternative models for roundabout capacity and performance estimation
- An Assessment of the Highway Capacity Manual 2010 Roundabout Capacity Model

Table comparing the features three well-known analytical models of roundabout capacity:

USA: HCM 2010 (Highway Capacity Manual 2010) model

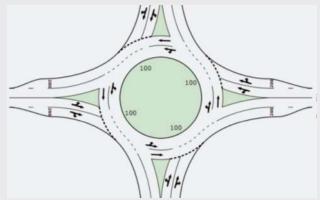
Australia: SIDRA INTERSECTION model, and

UK: TRL (linear regression) model (RODEL /ARCADY).

These models have some common features as well as significant differences.

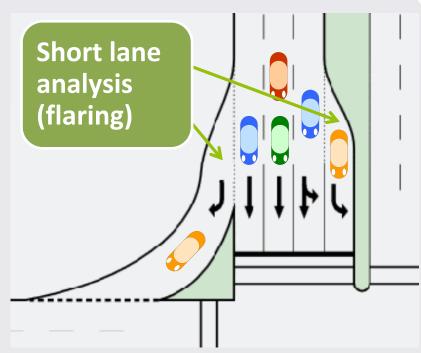


LANE – BASED method



- Lane flows
- Unequal lane use
- De facto exclusive lanes
- Approach short lanes
- Exit short lanes (lane use effects)
- Roundabout circulating lane use

These cannot be modelled using an APPROACH-BASED method



Effectiveness of flaring (short lanes) depends on flow conditions



Observed at UK Roundabouts

Lane use at flared approaches (short lanes) depends on flows



These cannot be modelled using an APPROACH-BASED method

Unequal lane utilisation





Observed at UK Roundabouts

Continuous lane without island causing lane underutilisation on another approach



This cannot be modelled using an APPROACH-BASED method



Observed at UK Roundabouts

Mini roundabouts operate effectively as all-way give-way control

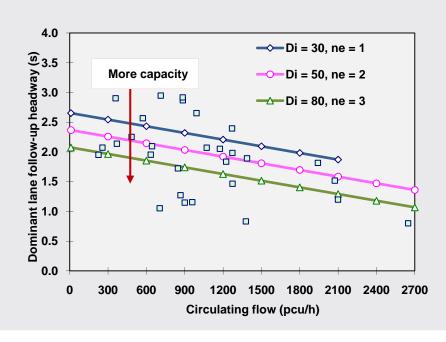


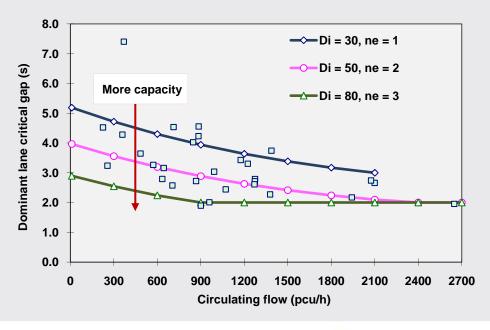




GEOMETRY and DRIVER BEHAVIOUR

Follow-up Headway and Critical Gap values decrease with increased flow rates, and depend on roundabout geometry







GEOMETRY parameters that affect capacity

- inscribed diameter
- number of entry lanes
- average entry lane width
- number of circulating lanes
- entry radius
- entry angle
- "flaring" as short lanes
- bypass (slip and continuous) lanes



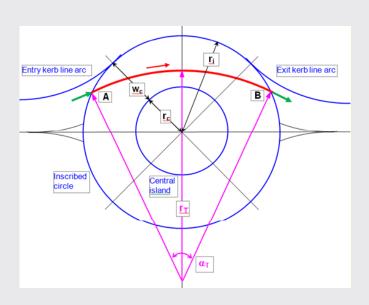
FLOW parameters that affect capacity

- circulating flow rate:
 increased values decrease the follow-up headway and critical gap
- circulating stream bunching characteristics (circulating lane flows)
- origin-destination flow patterns and queuing on approach lanes (for modelling unbalanced flow conditions)
- ratio of arrival flow to circulating flow
- ratio of dominant lane flow rate to subdominant lane flow rate
- heavy vehicles in the entry lane and circulating lane
- Environment Factor (general calibration parameter)



Roundabout model - LOW DEMAND

SIDRA INTERSECTION models negotiation radius, speed and distance allowing for path smoothing by drivers







Roundabout model – HIGH DEMAND

SIDRA INTERSECTION identifies congestion caused by heavy circulating flows especially with **UNBALANCED** flow patterns

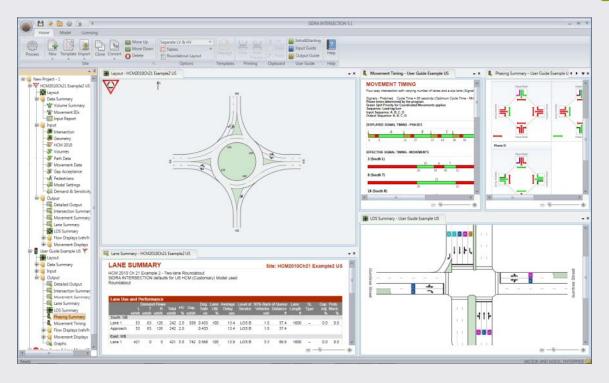




SIDRA ⇒ aaSIDRA ⇒ SIDRA INTERSECTION

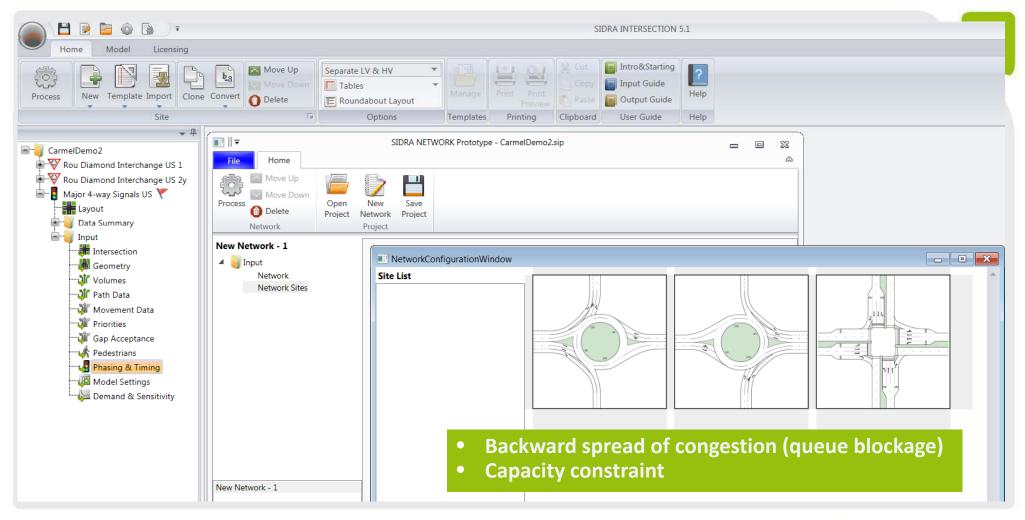


- First released in 1984
- SIDRA INTERSECTION 5.1 released two months ago





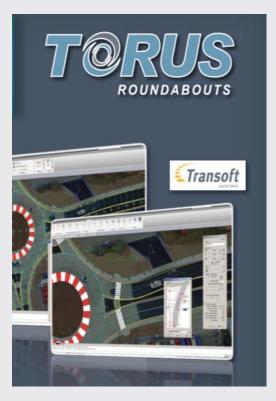
SIDRA NETWORK >> SIDRA INTERSECTION 6.0



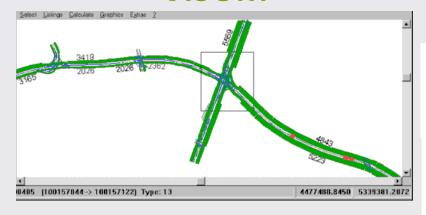


SIDRA interlinking with major software packages

Transoft Solutions



VISUM



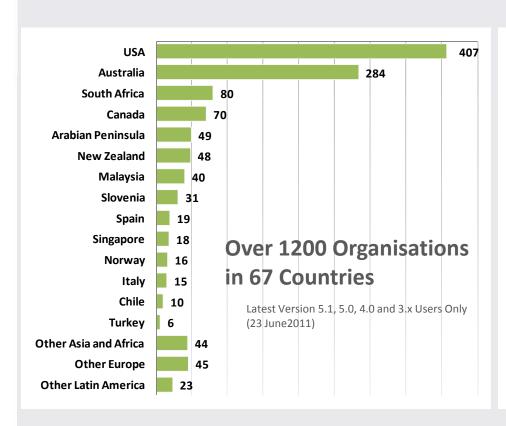


VISSIM





SIDRA INTERSECTION Users



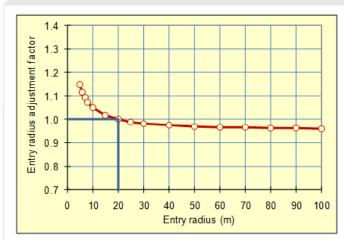


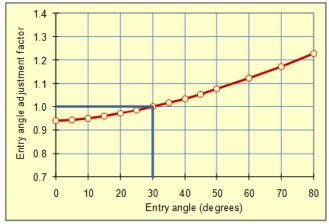


New Research



Entry Radius and Entry Angle in *SIDRA Standard* model (SIDRA INTERSECTION 5.1)





$$f_r = 0.95 + 1 / r_e$$

$$f_a = 0.94 + 0.00026 / \phi_e^{1.6}$$

$$r_e \text{ is the entry radius (m)}$$

$$\phi_e \text{ is the entry angle (degrees)}$$
Customary units:

 $f_r = 0.95 + 3.28 / r_e$ r_e is the entry radius (ft)

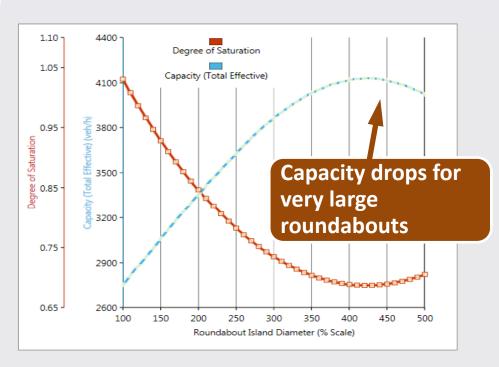
The entry radius and entry angle factors in SIDRA Standard and UK TRL models

r _e (m)	r _e (ft)	φ _e (degrees)	UK TRL	SIDRA Standard	
5	16	70	1.40	1.35	
10	33	60	1.18	1.18	
20	66	45	1.05	1.05	
30	98	35	1.00	1.00	
40	131	30	0.98	0.98	
60	197	15	0.92	0.93	
80	262	5	0.89	0.91	
100	328	0	0.87	0.90	

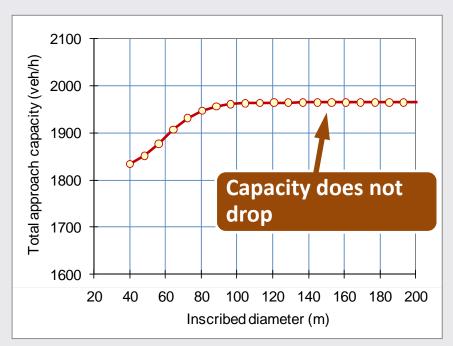


Roundabout Size: Inscribed Diameter

SIDRA Standard model

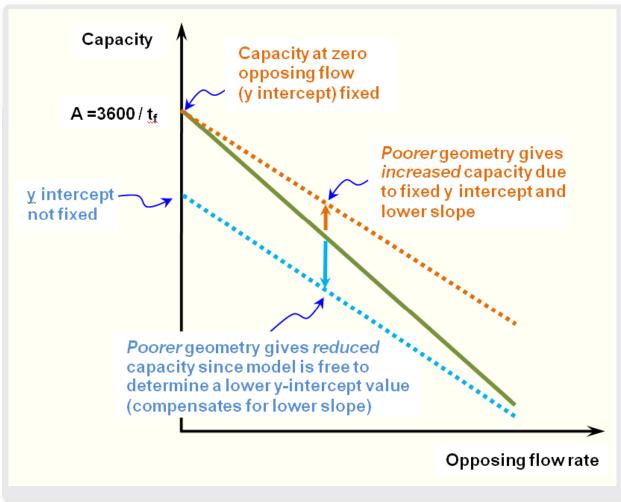


UK TRL model (RODEL, ARCADY)





Model calibration issue with the TRL model (RODEL, ARCADY)



A "fatal flaw": when the y-intercept is fixed:

Capacity decreases with improved geometry (increased entry radius, decreased entry angle, etc) if the capacity at zero circulating flow (y intercept) is fixed.

Refer to: LENTERS, M. and RUDY, C. (2010). HCM Roundabout Capacity Methods and Alternative Capacity Models. ITE Journal, 80 (7), pp. 22-27.



Roundabout Metering Signals CASE STUDY: Nepean Hwy – McDonald St, Melbourne, Australia, AM Peak



AKÇELIK, R. (2011). Roundabout metering signals: capacity, performance and timing. Paper presented at the 6th International Symposium on Highway Capacity and Quality of Service, Transportation Research Board, Stockholm, Sweden.







Metering Signals Project: Akcelik & Associates for VIC ROADS

A major project was undertaken by Akcelik & Associates for VIC ROADS, the state transport authority in Victoria, to investigate the performance of roundabouts with metering signals in Melbourne, Australia.

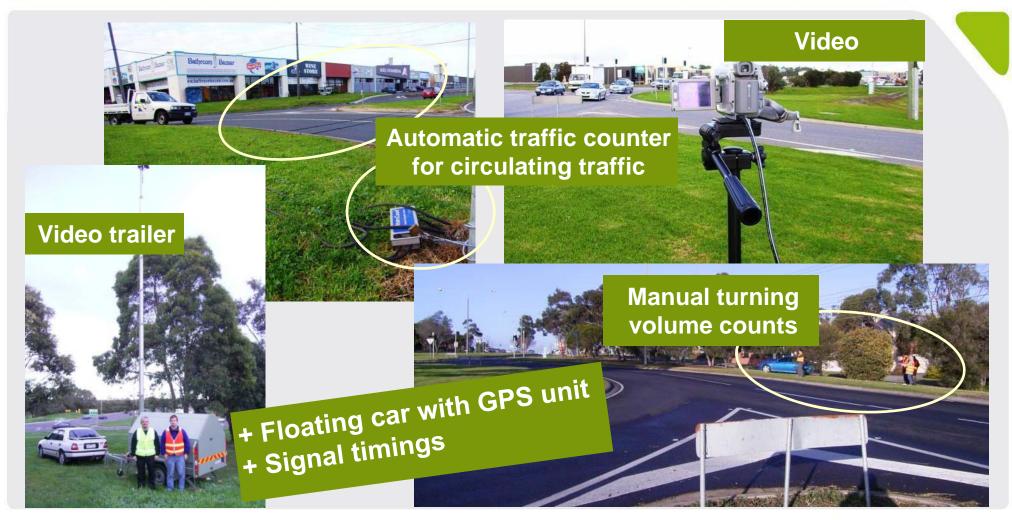
The project included comprehensive surveys of traffic and driver behaviour at roundabouts with metering signals.

Twenty roundabouts with metering signals were considered as candidates for the project. Five multi-lane roundabout sites were chosen for surveys (1 to 3 circulating lanes).

The peak 15-min intersection volumes at these sites were in the range approximately 3300 to 6000 veh/h.



Metering Signals Project - A&A for VIC ROADS: Surveys



Metering Signals Project - A&A for VIC ROADS

The roundabouts covered by the VIC ROADS project are shown in following slides.

For further information:

AKÇELIK, R. (2008). An investigation of the performance of roundabouts with metering signals. Paper presented at the National Roundabout Conference, Transportation Research Board, Kansas City, MO, USA.

www.sidrasolutions.com/software_downloads_articles.aspx



Metering Signals Project - A&A for VIC ROADS: Findings

	Circulating Traffic		Entry Lane Traffic		Measured		Estimated			
	Flow Rate (veh/h)	HVs	Flow Rate (veh/h)	HVs	Critical Gap (s)	Follow-up Headway (s)	Critical Gap (s)	Follow-up Headway (s)		
Metered and controlling approaches together										
Minimum	134	0.0%	174	0.0%	2.13	1.44	2.17	1.43		
15 th percentile	356	1.1%	288	0.7%	2.79	2.02	2.54	1.84		
Mean	674	4.6%	677	3.4%	3.20	2.30	3.37	2.10		
85 th percentile	984	8.6%	985	6.7%	3.60	2.59	3.98	2.38		
Maximum	1365	14.5%	1130	12.5%	4.31	3.25	4.83	3.11		



Unbalanced flows and metering signals: Mickleham Rd - Broadmeadows Rd, AM Peak





Boundary Road / Governor Road, PM Peak



Greensborough Bypass / Diamond Creek Rd, AM Peak





South Gippsland Hwy / Pound Rd, PM Peak



South Gippsland Hwy / Pound Rd, PM Peak



Fully signalised treatment

http://vicroads.vic.gov.au/Home/RoadProjects/MelbourneRoadProjects/SouthEasternSuburbs/PoundRoadDandenongSouth.htm







END OF PRESENTATION

